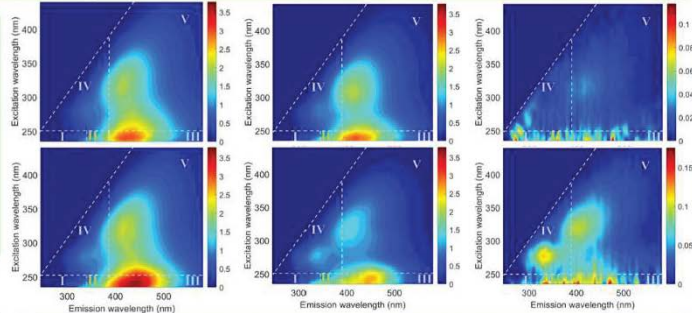
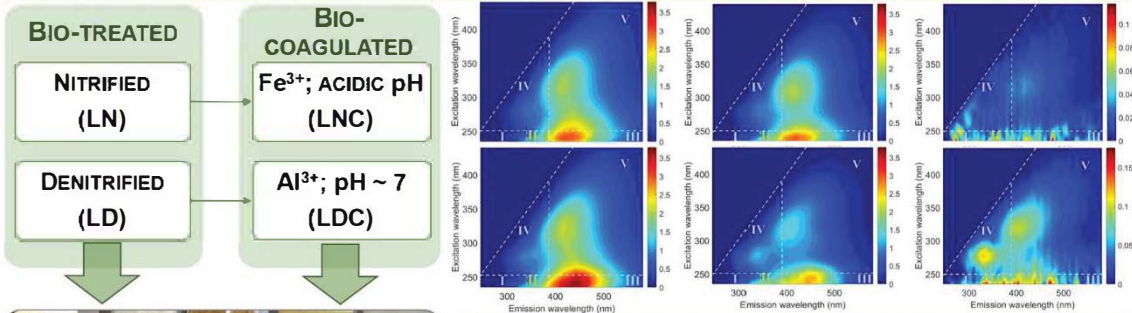


**LANDFILL LEACHATE – O<sub>3</sub> AND O<sub>3</sub>/UVC APPLIED AT DIFFERENT STAGES**



**ORGANIC AND NITROGEN LEGAL COMPLIANCE:**  
 LD → LDC → O<sub>3</sub>/UVC → Bio Oxidation → Direct Discharge

**OPERATION COSTS:**  
 0.67 + 1.23 + 6.89 + 0.06 = 8.6 €/m<sup>3</sup>

**How does the pre-treatment of landfill leachate impact the  
performance of O<sub>3</sub> and O<sub>3</sub>/UVC processes?**

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## 1 **Abstract**

2 In this study, O<sub>3</sub> and O<sub>3</sub>/UVC processes were evaluated for the treatment of landfill  
3 leachate after biological nitrification/denitrification, coagulation, or their combinations.  
4 The O<sub>3</sub>-driven stage efficiency was assessed by the removal of color, organic matter  
5 (dissolved organic carbon (DOC) and chemical oxygen demand (COD)), and  
6 biodegradability increase (Zahn-Wellens test). Also, fluorescence excitation-emission  
7 matrix (EEM) and size exclusion chromatography coupled with OC detector (SEC-  
8 OCD) analyzes were carried out for each strategy. The bio-nitrified-leachate (L<sub>N</sub>) was  
9 not efficiently mineralized during the O<sub>3</sub>-driven processes since the high nitrites content  
10 consumed ozone rapidly. In turn, carbonate/bicarbonate ions impaired the oxidation of  
11 the bio-denitrified-leachate (L<sub>D</sub>), scavenging hydroxyl radicals (HO•) and inhibiting the  
12 O<sub>3</sub> decomposition. For both bio-leachates, only O<sub>3</sub>/UVC significantly enhanced the  
13 effluent biodegradability (>70%), but COD legal compliance was not reached. EEM and  
14 SEC-OCD results revealed differences in the organic matter composition between the  
15 nitrified-coagulated-leachate (L<sub>NC</sub>) and denitrified-coagulated-leachate (L<sub>DC</sub>).  
16 Nonetheless, the amount of DOC and COD removed per gram of ozone was similar for  
17 both. Cost estimation indicates the O<sub>3</sub>-driven stage as the costliest among the treatment  
18 processes, while coagulation substantially reduced the cost of the following ozonation.  
19 Thus, the best treatment train strategy comprised L<sub>DC</sub> (with methanol addition for  
20 denitrification and coagulated with 300 mg Al<sup>3+</sup>/L, without pH adjustment), followed by  
21 O<sub>3</sub>/UVC (transferred ozone dose of 2.1 g O<sub>3</sub>/L and 12.2 kJ<sub>UVC</sub>/L) and final biological  
22 oxidation, allowed legal compliance for direct discharge (for organic and nitrogen  
23 parameters) with an estimated cost of 8.9 €/m<sup>3</sup> (O<sub>3</sub>/UVC stage counting for 6.9 €/m<sup>3</sup>).

24 **Keywords:** Mature landfill leachate; Ozonation; UVC radiation; Biodegradability;  
25 Dissolved organic matter; Operating costs.

## 26 **1. Introduction**

27 Solid waste disposal in sanitary landfills is the most common method used worldwide  
28 for waste management (Kaza et al., 2018). An inevitable consequence of this practice is  
29 the generation of leachate, generally known as high-strength wastewater, particularly  
30 difficult to handle (Christensen et al., 1989; Renou et al., 2008). Leachate from mature  
31 landfills (> 10 years) is usually characterized by (Bhalla et al., 2013; Renou et al.,  
32 2008): (i) high ammonium content; (ii) low biodegradability; and (iii) high fraction of  
33 refractory and large organic molecules, such as humic and fulvic acids (also responsible  
34 for the typical leachate dark brown color). If not properly treated, the discharge of  
35 landfill leachate into the environment poses serious risks, as it may cause a deep impact  
36 on soil permeability and contamination of surface and groundwater (Vithanage et al.,  
37 2014).

38 Over the last decade, several research efforts have been made combining different  
39 treatment processes (biological and/or physicochemical) with advanced oxidation  
40 processes (AOPs) as a strategy to effectively treat landfill leachates, such as: (i)  
41 sequencing batch biofilm reactor (SBBR) + electro-Fenton (EF) (Zhang et al., 2014);  
42 (ii) activated sludge biological oxidation (ASBO) + coagulation/sedimentation (C/S) +  
43 EF, photo-Fenton (PF), or photo-electro-Fenton (PEF) (Moreira et al., 2015; Silva et al.,  
44 2017); (iii) SBB granular reactor + solar PF, electrolysis, H<sub>2</sub>O<sub>2</sub>, H<sub>2</sub>O<sub>2</sub>/UV or O<sub>3</sub> (Del  
45 Moro et al., 2016; Pastore et al., 2018); (iii) autotrophic nitrogen removal (ANR)  
46 combine with C/S, O<sub>3</sub> or Fenton, with pre- or post-treated by granular activated carbon  
47 (GAC) (Gao et al., 2015; Oloibiri et al., 2017a). However, those studies mainly target  
48 the removal of chemical oxygen demand (COD) and only few consider the nitrogen  
49 content (usually the ammonia content and not the total nitrogen (TN)). So, in view of  
50 simultaneous compliance for organic and nitrogen compounds legal parameters, V.

51 Vilar and co-workers proposed and tested a treatment train strategy that encompassed  
52 biological nitrification/denitrification, coagulation with iron salts at acidic pH, followed  
53 by PF oxidation and final biological oxidation (Gomes et al., 2019b; Silva et al., 2017).  
54 Also, a new artificial photoreactor (FluHelik) was developed for the advanced treatment  
55 of effluents with low transmissibility, like leachates. This strategy was tested at full-  
56 scale, where the FluHelik's design (4 photoreactors connected in series) enabled a  
57 flexible and compact solution for the photo-treatment of 20 m<sup>3</sup> of leachate and high  
58 performance was reported (Gomes et al., 2019a). Despite the promising results, some  
59 operational difficulties were highlighted regarding the PF stage, namely: (i) requires  
60 tight alkalinity control of the biological nitrification/denitrification stage; (ii) obliges the  
61 addition of chemicals, which may jeopardize compliance with legal discharge  
62 parameters (namely for sulfate ions and total iron); (iii) requires additional treatment  
63 time for iron-catalyst sedimentation; and (iv) produces iron-sludge. Considering these  
64 drawbacks, the usage of O<sub>3</sub>-driven processes as an alternative to the PF stage was  
65 proposed.

66 Ozonation processes can be attractive for the treatment of landfill leachates, namely  
67 because ozone presents: (i) high oxidative power; (ii) great efficiency in color removal;  
68 (iii) high reactivity and selectivity towards electron-rich moieties of compounds; and  
69 (iv) the possibility to be combined with H<sub>2</sub>O<sub>2</sub> and/or UVC radiation, which favors the  
70 production of hydroxyl radicals (HO<sup>•</sup>). These processes have been applied either to the  
71 raw leachate, mainly as a pre-treatment (Asaithambi et al., 2017; Cortez et al., 2011;  
72 Tizaoui et al., 2007), or later as a polishing step (Amaral-Silva et al., 2016; Cortez et al.,  
73 2010; Ntampou et al., 2006) prior to discharge in the environment. Beyond removing  
74 contaminants and reducing the organic load, several ozone-driven processes (O<sub>3</sub>/UVC,  
75 O<sub>3</sub>/H<sub>2</sub>O<sub>2</sub>, O<sub>3</sub>/UVC/H<sub>2</sub>O<sub>2</sub>, catalytic or electro-catalytic-ozonation) have also proven to

76 enhance the biodegradability of landfill leachate (Ghahrchi and Rezaee, 2020; 2021).  
77 This is particularly relevant to outline a strategy for leachate treatment, as the  
78 combination of an ozone-driven process followed by a biological stage is expected to  
79 reduce the required ozone dose (thus, its costs) to reach legal compliance for direct  
80 discharge. In view of this, our previous research work (Gomes et al., 2020) focused on  
81 the application of different ozone-driven processes ( $O_3$ ,  $O_3/UVC$ ,  $O_3/H_2O_2$  and  
82  $O_3/UVC/H_2O_2$ ), testing different initial pH values, ozone inlet concentrations, ozone  
83 rate flows and reactor system setups, on the treatment of a biologically nitrified and  
84 coagulated leachate. The main findings allowed to (i) establish the best operational  
85 conditions and system setup (in this case, a FluHelik coupled with a bubble column,  
86 with a Venturi injector at the inlet of the photoreactor); and (ii) demonstrate that the  
87 recalcitrant organic matter was successfully oxidized by  $O_3$  and, in particular, by  
88  $O_3/UVC$ , making it suitable for a following biological oxidation (both treatments  
89 reached COD values  $< 150$  mg/L, Portuguese legal limit for direct discharge (236/98,  
90 1998), at the end of a Zahn-Wellens biodegradability test). Although COD legal  
91 compliance was met, the high TN content ( $\sim 0.56$  g/L) of the leachate was not addressed,  
92 and many countries present regulatory limits for both parameters (Gomes et al., 2019b).  
93 With the purpose to integrate an  $O_3$ -driven stage into a treatment train for mature urban  
94 landfill leachates and underlying compliance with legal discharge limits, this work  
95 investigates the performance of  $O_3$  and  $O_3/UVC$  processes applied to leachate pre-  
96 treated with different technologies (including biological nitrogen removal and  
97 coagulation processes). The efficiency of the ozone-driven stage was evaluated by the  
98 removal of organic matter (in terms of dissolved organic carbon (DOC) and COD) and  
99 color, as well as by the ability to enhance the effluent biodegradability (in view of a  
100 complete treatment train containing a subsequent and final biological oxidation). For a

101 deeper understanding of the nature and fingerprints of the dissolved organic matter  
102 (DOM) in (treated) leachates, for each treatment strategy, the changes in the  
103 characteristics of organic matter in leachate samples were evaluated, by means of  
104 fluorescence excitation-emission matrix (EEM) spectroscopy and size exclusion  
105 chromatography coupled with organic carbon detector (SEC-OCD). Finally, estimations  
106 for the treatment costs considering 6 possible treatment train scenarios ((1) bio-  
107 nitrification + O<sub>3</sub>/UVC + bio-denitrification; (2) bio-N removal + O<sub>3</sub>/UVC + bio-  
108 oxidation, (3;4) bio-nitrification + iron-acidic coagulation (no nitrites) + O<sub>3</sub> or O<sub>3</sub>/UVC  
109 + bio-denitrification, and (5;6) bio-N removal + aluminum-neutral coagulation + O<sub>3</sub> or  
110 O<sub>3</sub>/UVC + bio-oxidation) were performed.

## 111 **2. Materials and methods**

### 112 *2.1. Urban landfill leachate*

113 The mature urban leachate used in this work was pre-treated by: (i) biological  
114 nitrification (L<sub>N</sub>); (ii) biological nitrification/denitrification (L<sub>D</sub>); (iii) biological  
115 nitrification followed by coagulation (L<sub>NC</sub>); and (iv) biological  
116 nitrification/denitrification followed by coagulation (L<sub>DC</sub>). The main physicochemical  
117 characteristics of each pre-treated leachate are presented in Table 1.

#### 118 **Insert Table 1**

119 Both nitrified leachates (L<sub>N</sub> and L<sub>NC</sub>) were collected at a leachate treatment plant  
120 installed in a 20-year-old municipal landfill located in northern Portugal. L<sub>N</sub> was  
121 collected after the aerated biological treatment stage (temperature and pH not  
122 controlled; hot air compressor and fine bubble aeration, 0.5 mg/L < dissolved oxygen <  
123 1.0 mg/L, to promote nitrification), while L<sub>NC</sub> was collected at the end of the following  
124 coagulation stage (coagulant dose of 240 mg Fe<sup>3+</sup>/L and pH ~4). Both pre-treated

125 leachates were transported to the laboratory and stored at 4°C until use. Before storage,  
 126  $L_{NC}$  was aerated (air pump at 3000 L/h, for 4h) for the conversion of nitrites to nitrates.  
 127 In order to obtain the denitrified leachates ( $L_D$  and  $L_{DC}$ ), part of the collected  $L_N$  was  
 128 placed in a biological reactor (10 L capacity) with biomass previously adapted, under  
 129 anoxic conditions and mechanical agitation, at room temperature (22-25°C). Methanol  
 130 was added as external carbon source for denitrification (2.5 mg  $CH_3OH/mg$   $N-NO_2^-$ ).  
 131 After denitrification, part of  $L_D$  was used for coagulation tests with aluminum sulfate  
 132 ( $Al_2(SO_4)_3$ ), by means of *jar test*, without pH adjustment (further details can be found at  
 133 *Supplementary Material* file).

## 134 2.2. Experimental setup

135 The lab-scale system consisted of a FluHelik photoreactor connected to a bubble  
 136 column reactor ( $\varnothing_{inner} = 73$  mm; maximum fluid column height of 370 mm), with  
 137 leachate recirculation (ISMATEC BVP-Z pump, flow rate ( $Q_l$ ) of 75 L/h) amongst the  
 138 reactors (Figure 1). The tests were conducted in semi-batch mode employing a Venturi  
 139 to inject a continuous stream of  $O_3/O_2$ -gas mixture (ozone generator BMT 802N)  
 140 upstream the photoreactor. The gaseous flow rate was regulated by a digital mass flow  
 141 meter (Alicat Scientific). The ozone concentrations in the inlet and outlet of the system  
 142 were measured (ozone analyser BMT 964), and the transferred ozone dose ( $OD_T$ ) was  
 143 calculated according to Eq. (1).

$$144 \quad OD_T = \frac{Q_g}{V_L} \int_0^t (C_{O_3, I-g} - C_{O_3, O-g}) dt \quad (1)$$

145 with  $OD_T$  (mg  $O_3/L$  effluent), calculated by integrating the difference between the  
 146 constant inlet ozone concentration ( $C_{O_3, I-g}$ ) and the outlet/off-gas concentration ( $C_{O_3, O-g}$ )  
 147 during the applied time interval.  $Q_g$  (L/min) is the applied gas flow rate and  $V_L$  (L) is the  
 148 volume of leachate in the reactor.



149 The ozone leaving the system was vented through a catalytic ozone destruction unit  
150 (Heated Catalyst BMT) and bubbled into Woulff bottles containing 2% KI solution. For  
151 the O<sub>3</sub>/UVC process, a UVC lamp (Philips TUV 11W (bio-treated) or 6W (bio-  
152 coagulated), model G6 T5, with the corresponding radiant power of 2.48 J/s and 1.7 J/s)  
153 was placed inside the FluHelik.

### 154 *2.3. Experimental procedure*

155 In view of the proposed objectives, for each pre-treated leachate, experiments were  
156 carried out with O<sub>3</sub> and O<sub>3</sub>/UVC processes. All tests were performed using the  
157 following experimental conditions: initial leachate pH of 9.0 (using NaOH), inlet ozone  
158 dose of 18 mg O<sub>3</sub>/min (inlet O<sub>3</sub> concentration set at 180 mg/L and gas flow rate at 0.1  
159 L/min). Each treatment test was applied to 1.5 L of pre-treated leachate for (i) 12h and  
160 10h, for O<sub>3</sub> and O<sub>3</sub>/UVC processes, respectively, for LN and LD effluents; and (ii) 3h,  
161 for all tests applied to L<sub>NC</sub> and L<sub>DC</sub>. During the tests, samples were collected  
162 periodically for analytical characterization, at a sampling point located between the  
163 FluHelik and the BC. To eliminate residual dissolved ozone, sample vials were  
164 transferred to a water bath at 50 °C, for 15 min.

### 165 *2.4. Analytical determinations*

166 Chloride (Cl<sup>-</sup>), nitrite (NO<sub>2</sub><sup>-</sup>), sulfate (SO<sub>4</sub><sup>2-</sup>), and nitrate (NO<sub>3</sub><sup>-</sup>) were quantified by ion  
167 chromatography using a Dionex ICS-2100 apparatus. Heavy metals (Al, Cr, Cu, Fe, Ni  
168 and Pb) were quantified by inductively coupled plasma optical emission spectrometry  
169 (ICP-OES), using a Thermo iCAP 7000 equipment, after sample digestion with nitric  
170 acid. The analysis of COD was made according to the dichromate closed reflux method  
171 (Clesceri et al., 2005). The DOC determination was carried out using a Shimadzu TOC-  
172 V<sub>CSN</sub> analyzer (with previous filtration of the sample using 0.45 µm Nylon membrane  
173 filters). The UV-Vis spectrum (after filtration and dilution) was measured in a scan

174 range of 200-600 nm (1 nm interval), using a UV-Vis spectrometer (VWR, UV-  
175 6300PC) and 1 cm quartz cuvette. Before and after each treatment stage the leachate's  
176 biodegradability was evaluated by the Zahn-Wellens test (OECD, 1992). Ultrapure and  
177 demineralized water were produced by a Millipore system (Direct-Q®) and a reverse  
178 osmosis system (Panice), respectively.

179 Fluorescence and absorbance spectra were measured using a Horiba Aqualog  
180 fluorometer. Fluorescence was scanned with excitation wavelengths from 225 to 450  
181 nm in 5 nm increments and emission wavelengths from 250 to 580 nm in 1 nm  
182 increments. The inner filter was corrected using the measured absorbance spectra as  
183 described in Park and Snyder (2018). The regional integration and total fluorescence  
184 (TF) were calculated as described in Chen et al. (2003).

185 Dissolved organic matter (DOM) was characterized by size exclusion chromatography  
186 coupled with organic carbon detector (SEC-OCD) using a hydroxylated methacrylic  
187 polymer column (TOYOPEARL® HW-50S, Tosoh Bioscience LLC; 21 mm x 250  
188 mm), high-performance liquid chromatography (HPLC) (Agilent 1290) hyphenated  
189 with an OCD (Suez GE Sievers M9 TOC analyzer).

### 190 *2.5. Treatment cost analysis*

191 For the biological stages, to estimate the energy cost for aeration and the cost of the  
192 external carbon donor, theoretical values for oxygen and COD consumption were  
193 employed. For the coagulation and ozone-driven stages, the operating costs were  
194 calculated taking into account the experimental conditions and data obtained, namely:  
195 (i) type and concentration of coagulant; (ii) amount of acid or base required for the pH  
196 adjustments; (iii) gas flow rate and ozone concentration and (iv) time required to  
197 achieve 85% ( $L_N$  and  $L_D$ ) or 60% ( $L_{NC}$  and  $L_{DC}$ ) of COD reduction (established  
198 according to the results obtained and calculated on the basis of the pseudo-first-order

199 rate constants). Furthermore, the following energy and reagents prices were considered:  
200 (i) 0.1276 €/kWh, energy market price in Portugal for industrial applications; (ii) 4.5 kg  
201 O<sub>2</sub>/kWh for aeration carried out by fine bubble diffusers (Tchobanoglous et al., 2003);  
202 15 kWh/kg O<sub>3</sub> for a corona discharge ozone generator fed with pure oxygen; (iv) 11W  
203 (L<sub>N</sub> and L<sub>D</sub>) or 6 W (L<sub>NC</sub> and L<sub>DC</sub>) for UVC lamp; (v) 0.48 €/kg for methanol; (vi) 0.08  
204 €/m<sup>3</sup> for O<sub>2</sub>; (vii) 0.24 €/kg for FeCl<sub>3</sub> 40% (w/w); (viii) 0.19 €/kg for Al<sub>2</sub>(SO<sub>4</sub>)<sub>3</sub> 48%  
205 (w/w), (ix) 0.16 €/kg for NaOH 30% (w/w), and (x) 0.10 €/kg for H<sub>2</sub>SO<sub>4</sub> 98% (w/w).  
206 Regarding the sludge treatment and disposal costs, based on previous work (Gomes et  
207 al., 2019a) an average value of 0.48 €/m<sup>3</sup> was assumed.

### 208 **3. Results and discussion**

#### 209 *3.1. Bio-treated leachate samples*

210 According to the characteristics of the two bio-treated leachates (L<sub>N</sub> and L<sub>D</sub>) showed in  
211 Table 1, main differences are found for pH, inorganic carbon (DIC), and nitrogen  
212 content, which are related to the distinct biological metabolic processes involved in the  
213 nitrification and denitrification of the leachate. For biological nitrification (in this case  
214 partial nitrification, i.e., ammonium oxidation to nitrite), the autotrophic nitrifying  
215 bacteria consumed the inorganic carbon, thus reducing alkalinity and pH (theoretically,  
216 7.14 mg of CaCO<sub>3</sub> are consumed per 1 mg of NH<sub>4</sub><sup>+</sup>-N oxidized) (U.S.EPA, 1993). In  
217 turn, heterotrophic bacteria in biological denitrification recovered part of the inorganic  
218 carbon, which raises alkalinity and pH (theoretically, each mg of NO<sub>2</sub><sup>-</sup>-N reduced to N<sub>2</sub>  
219 causes an alkalinity increase of 3.57 mg CaCO<sub>3</sub>) (U.S.EPA, 1993). Consequently, L<sub>N</sub>  
220 effluent presents lower pH and DIC values, when compared to L<sub>D</sub>, and the nitrogen  
221 content was mainly in the form of nitrite ions. Since L<sub>D</sub> effluent resulted from  
222 denitrification of L<sub>N</sub>, both presented similar concentration values for chloride and  
223 sulfate ions, as for some selected heavy metals (Table 1). Nonetheless, L<sub>D</sub> effluent had

224 slightly higher DOC and COD values, most likely attributed to some remnants of  
225 biodegradable organic matter from the addition of the external carbon donor. This was  
226 supported by the Zahn-Wellens biodegradability test results.

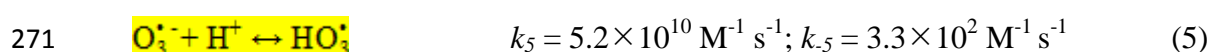
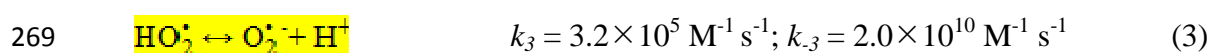
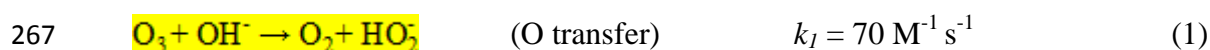
### 227 3.1.1. Efficiency of the ozone-driven processes

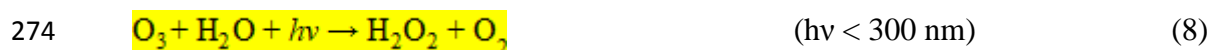
228 Taking the above, the effect of the L<sub>N</sub> and L<sub>D</sub> distinct features on the performance of O<sub>3</sub>  
229 and O<sub>3</sub>/UVC treatment was assessed. A first glance at the results (Table 2) suggests no  
230 significant differences between the efficiencies of the ozone-driven processes applied to  
231 L<sub>N</sub> and L<sub>D</sub>. For both bio-treated leachates, DOC and COD removals were very similar at  
232 the end of the ozonation treatment. Also, the combination of O<sub>3</sub>/UVC further increased  
233 the organic matter oxidation, with L<sub>D</sub> presenting 10% less mineralization than L<sub>N</sub>. A  
234 closer look at the organic matter removal profiles (Fig. 2) reveals specific features  
235 during the ozone-based treatments.

#### 236 **Insert Table 2**

237 For L<sub>N</sub>, there was an initial period up to about 7h, for O<sub>3</sub>, and 5h, for O<sub>3</sub>/UVC  
238 (corresponding to  $OD_T$  of 5.0 and 3.6 g O<sub>3</sub>/L, respectively), in which practically no  
239 mineralization was observed (Fig. 2 (a.1) and (a.2)). This induction period coincided  
240 with the oxidation of nitrites to nitrates ( $\text{NO}_2^- + \text{O}_3 \rightarrow \text{NO}_3^- + \text{O}_2$ ,  $k = 1.8 \times 10^5 \text{ M}^{-1} \text{ s}^{-1}$   
241 (Hoigné et al., 1985),  $k = 5.8 \times 10^5 \text{ M}^{-1} \text{ s}^{-1}$  (Liu et al., 2001)) and did not appear to  
242 affect COD reduction. A clear linear relation was obtained between the  $OD_T$  and the  
243 amount of nitrites oxidized (see *Supplementary Material*, Fig. SM-2), with an expected  
244 highest rate found for O<sub>3</sub>/UVC treatment. This high ozone consumption hindered  
245 mineralization that started as soon as all the nitrite was converted to nitrate. Afterward  
246 and until the end of the tests, the DOC reduction profile was followed by a significant  
247 drop in pH (down to pH 3.2 and 5.6 for O<sub>3</sub>-only and O<sub>3</sub>/UVC treatments, respectively,  
248 Fig. 2 (b.1) and (b.2)). This acidification may be associated with the oxidation of

249 complex organic compounds in the leachate with the expected formation of carboxylic  
 250 acids (Chys et al., 2015; Tizaoui et al., 2007), and with carbon dioxide and carbonic  
 251 acids resulting from the mineralization of organic compounds. It is known that pH  
 252 influences the oxidation pathways for ozonation processes (higher pH decomposes  
 253 ozone more rapidly, since hydroxide ion (OH<sup>-</sup>) is an initiator of the reaction chain  
 254 described in Equations 1 to 7, thus producing a greater amount of HO<sup>•</sup> radicals (Gardoni  
 255 et al., 2012)) and also plays an important role by influencing the concentrations of the  
 256 dissociated/non-dissociated forms of the leachate components. So, it is interesting to  
 257 note that the mineralization kinetic constants ( $k$ , min<sup>-1</sup>) obtained for L<sub>N</sub> became higher  
 258 when its buffer capacity was lost. This indicates that other mechanisms were occurring  
 259 in the medium, generating reactive species and contributing to the oxidation of the  
 260 organic matter. For instance, although (as mentioned) O<sub>3</sub> reacts with nitrite mainly to  
 261 form nitrate, peroxyxynitrite (2.6%) and HO<sup>•</sup> radicals (~ 8%) are also generated, and 50%  
 262 of peroxyxynitrite (stable at high pH) potentially gives rise to further HO<sup>•</sup> radicals when  
 263 the pH decreases to near neutral conditions (Naumov et al., 2010). Furthermore, in the  
 264 presence of UVC radiation, ozone is photolyzed to form hydrogen peroxide (H<sub>2</sub>O<sub>2</sub>),  
 265 which can itself be photolyzed generating HO<sup>•</sup> radicals or act as an initiator for the  
 266 ozone chain reaction mechanism to form HO<sup>•</sup> radicals (Equations 8 to 10).





277 **Insert Figure 2**

278 Although the DOC and COD removal of L<sub>N</sub> and L<sub>D</sub> at the longest exposure time were  
 279 similar to each other, L<sub>D</sub> exhibited higher attenuation for both parameters at earlier  
 280 exposure times. This trend in the treatment of L<sub>D</sub>, when compared to L<sub>N</sub>, can be justified  
 281 by (1) absence of nitrites (avoiding ozone consumption for nitrites oxidation into  
 282 nitrates during the first hours of treatment, making it available for organic matter  
 283 oxidation) and (2) pH buffer effect due to the higher alkalinity content (observed by the  
 284 less pronounced pH decay, Fig.1 (b.1) and (b.2)). The latter can cause both positive and  
 285 negative impacts on mineralization efficiency during ozonation. On the one hand, L<sub>D</sub>  
 286 alkalinity allowed to maintain higher pH values throughout the treatments (particularly  
 287 for O<sub>3</sub>/UVC), thus promoting the autocatalytic reaction that produces HO<sup>•</sup> radicals,  
 288 strong and non-specific oxidants that are responsible for mineralization (Gardoni et al.,  
 289 2012; Rosenfeldt et al., 2006); on the other hand, it is known that carbonate and  
 290 bicarbonate ions may act as HO<sup>•</sup> scavengers (as described in Equations 11 and 12),  
 291 hindering the organic matter oxidation (Miklos et al., 2018), and also as “inhibitors” in  
 292 the O<sub>3</sub> decomposition (Elovitz et al., 2000).



295 Ozone effectively reacts with chromophores. This was clearly seen by the color  
 296 evolution during the first hour of treatment (see Fig. SM-3). At the end of the ozone-  
 297 driven treatments, the intense brown color presented by both bio-treated leachate  
 298 decreased to a very slight yellow. The color values (for a 1:20 dilution) obtained at the

299 end of the treatments by  $O_3$  and  $O_3/UVC$  were, respectively, 28 and 9 units of Pt-Co, for  
300  $L_N$ , and 30 and 20 units of Pt-Co, for  $L_D$ . Another indicator that can be employed to  
301 assess the effectiveness of ozone-driven processes is the change in biodegradability  
302 (Chen et al., 2019). Therefore, prior to and after the ozonation treatments, the  
303 biodegradability of each leachate was evaluated by the Zahn-Wellens 28-day test and  
304 the following results were obtained: (i)  $L_N = 7\%$ , after  $O_3 = 51\%$  and  $O_3/UVC = 73\%$ ;  
305 and (ii)  $L_D = 11\%$ , after  $O_3 = 69\%$  and  $O_3/UVC = 80\%$ . Bearing in mind that for the  
306 effluent to be considered biodegradable, the organic carbon content must decrease by  
307 70% after the 28-day test (Pluciennik-Koropczuk and Myszograj, 2018; Silva et al.,  
308 2013), for both bio-treated leachates, this only occurred after treatment with  $O_3/UVC$ .  
309 Also, in view of a target COD value  $< 150$  mg/L (the legal limit for direct discharge), it  
310 is worth to mention that, in spite not achieved for any of the tested conditions, for the  
311  $O_3/UVC$  treatments, COD values of 205 and 167 mg/L were reached at the end of the  
312 biodegradability tests for  $L_N$  and  $L_D$ , respectively. Therefore, the treatment of bio-  
313 treated leachate with  $O_3$  alone will be disregarded as a possible treatment strategy.

### 314 *3.1.2. Organic matter characterization*

315 According to Chen et al. (2003), the fluorescence spectrum can be divided into five  
316 regions based on the type and location of fluorescent material: (i) region I – tyrosine-  
317 like aromatic protein ( $Ex < 250$  nm,  $Em < 330$  nm); (ii) region II – tryptophane-like  
318 aromatic protein ( $Ex < 250$  nm,  $330$  nm  $< Em < 380$  nm); (iii) region III – fulvic-like  
319 acids ( $Ex < 250$  nm,  $Em > 380$  nm); (iv) region IV – soluble microbial metabolic by-  
320 products ( $Ex > 250$  nm,  $Em < 380$  nm) and (v) region V – humic-like acids ( $Ex > 250$   
321 nm,  $Em > 380$  nm). Figure 3a shows the relative fluorescence intensities integrated at  
322 each fluorescent region. A similar distribution of relative fluorescence was observed for  
323  $L_N$  and  $L_D$ , inferring similar physicochemical characteristics of fluorophores. This can

324 be expected as  $L_D$  was obtained from the biological denitrification of  $L_N$  effluent. For  
325 both bio-treated effluents, 3D-EEM contours (Fig. 3 (b.1) and (b.2)) highlight two main  
326 regions responsible for more than 60% of total fluorescence intensity: (i) the fulvic acid-  
327 like matter (region III), representing 31.0% ( $L_N$ ) and 33.5% ( $L_D$ ), and (ii) humic acid-  
328 like matter (region V), corresponding to 35.5% ( $L_N$ ) and 33.1% ( $L_D$ ). This high fraction  
329 of fulvic- and humic-like acids is also an indication of the chemical stability of the  
330 leachate (Oloibiri et al., 2017b).

### 331 **Insert Figure 3**

332 After  $O_3/UVC$  treatment, the total fluorescence intensity largely decreased (95% and  
333 92% for  $L_N$  and  $L_D$ , respectively), but major differences can be visualized in the 3D-  
334 EEM for both tested leachates (Fig. 3 (b.1') and (b.2')). While for the  $L_N$ , the fulvic-  
335 and humic-like matter regions remained with high fluorescence (30.8% and 39.1%,  
336 respectively, of the total fluorescence), for  $L_D$ , the fluorescence of those regions  
337 significantly decreased. The region with greater fluorescence intensity (representing  
338 31.2% of the total fluorescence) was related to soluble microbial by product-like matter  
339 (region IV). Furthermore, for  $L_D$ , after  $O_3/UVC$  treatment, there was an increase of  
340 fluorescence related to protein-like substances (contrary to the treated  $L_N$ ), with regions  
341 I and II counting for 36.3% of the total fluorescence. Together, these features of the  
342 treated  $L_D$  are likely responsible for its higher biodegradability.

343 Size exclusion chromatography combined with organic carbon detection (SEC-OCD) is  
344 a well-proven method for separating the pool of DOM into main fractions of different  
345 sizes and chemical functions and quantifying these on the basis of organic carbon  
346 (Huber et al., 2011). Observing the SEC-OCD chromatograms (Fig. 3 (c)), for both bio-  
347 treated leachates it is possible to verify the predominance of humic substances, with no  
348 noticeable peaks for lower molecular weight matter, such as building blocks and low



349 molecular weight acids/neutrals. After O<sub>3</sub>/UVC treatment, the peak area related to  
350 humic substances decreased significantly, particularly for L<sub>D</sub> (72.7% decay vs. 58.9%  
351 for L<sub>N</sub>). Also, there was a shift of retention time at which the maximum OCD intensity  
352 was observed, suggesting lower molecular weight of the humic substances remaining.

### 353 *3.2 Bio-coagulated leachates*

354 The bio-coagulated leachates (L<sub>NC</sub> and L<sub>DC</sub>) were coagulated, considering the distinct  
355 characteristics of their corresponding bio-treated leachates (Table 1). L<sub>N</sub> presents low  
356 alkalinity (as discussed above), which allows its acidification (i.e., sulfuric acid  
357 addition) without compromising the discharge limit for sulfate ions (Table 1). So, as  
358 proposed by different authors (Amokrane et al., 1997; Castrillón et al., 2010; Daud et  
359 al., 2012; Vedrenne et al., 2012) and tested on previous works with this type of leachate  
360 (Gomes et al., 2019a; Gomes et al., 2019b; Silva et al., 2017), L<sub>NC</sub> results from  
361 coagulation using ferric salts at acidic conditions (240 mg Fe<sup>3+</sup>/L and pH ~4). Attention  
362 should be drawn that the L<sub>NC</sub> effluent was collected in the landfill plant and does not  
363 correspond to the direct coagulation of the L<sub>N</sub> used for testing. Nonetheless, from  
364 previous coagulation tests under similar conditions, also using nitrified leachate from  
365 the same landfill (Gomes et al., 2019a; Silva et al., 2017), the performance of the  
366 coagulation stage to obtain L<sub>NC</sub> can be assumed between 47% to 60% removal for both  
367 DOC and COD parameters. Note that aeration was afterward applied to oxidize nitrites  
368 to nitrates (Table 1), eliminating its interference in the ozonation reactions towards the  
369 bio-recalcitrant organic content of the L<sub>NC</sub>.

370 In turn, to take advantage of the basic pH and buffering capacity (due to the recovery of  
371 alkalinity by bio-denitrification) of L<sub>D</sub>, and to attain a final coagulated effluent with  
372 near-neutral pH conditions, coagulation with aluminum salts at natural pH was tested.  
373 Considering the *jar-test* results (Table SM-1) and the sulfate legal limit compliance (due

374 to the addition of  $\text{Al}_2(\text{SO}_4)_3$  as coagulant), a coagulant dose of  $300 \text{ mg Al}^{3+}/\text{L}$  was  
375 selected. Using the natural pH of the  $L_D$  effluent, the selected coagulant dose allowed  
376 50.5% and 54.4% of DOC and COD removal, 63.3% of turbidity reduction, and a final  
377 pH of 6.2 (Table SM-1).

### 378 3.2.1. Efficiency of the ozone-driven processes

379 For  $L_{NC}$ , as a result of the absence of nitrites, more oxidant species were available to  
380 react with the organic matter and, unlike  $L_N$ , not only COD but also DOC has decreased  
381 continuously since the beginning of the ozone-driven treatments (Fig. 4 (a.1) and (a.2)).  
382 Additionally, for both  $\text{O}_3$  and  $\text{O}_3/\text{UVC}$  treatments, the  $L_{NC}$  presented higher percentages  
383 of DOC and COD removal when compared to the  $L_{DC}$  (Table 2). However, it can be  
384 misleading, as attention should be given to the differences of the organic content  
385 between the bio-coagulated leachates (lower organic content of  $L_{NC}$  when compared to  
386  $L_{DC}$ , see Table 1). Thus, although  $L_{NC}$  presents higher pseudo-first-order rate constants  
387 ( $k, \text{min}^{-1}$ ) for DOC and COD removal, it is possible to verify that the amount of DOC  
388 and COD removed per gram of ozone is very similar for both bio-coagulated leachate  
389 effluents. This feature was valid for the two ozone-driven processes tested. For the bio-  
390 treated leachates, the pH decay for  $L_{DC}$  was less pronounced due to its higher alkalinity  
391 content (buffer effect) (Fig. 4 (b.1) and (b.2)). Also, for both bio-coagulated leachates,  
392 pH decay was lower for the  $\text{O}_3/\text{UVC}$  process.

### 393 **Insert Figure 4**

394 The color values (for a 1:20 dilution) obtained at the end of the treatments (3h) by  $\text{O}_3$   
395 and  $\text{O}_3/\text{UVC}$  were, respectively, 13 and 9 units of Pt-Co, for  $L_{NC}$ , and 12 and 9 units of  
396 Pt-Co, for  $L_{DC}$  (Fig. SM-5). In respect to the biodegradability before and after ozone-  
397 driven treatments, the following results were obtained: (i)  $L_{NC} = 17\%$ , and after  $\text{O}_3 =$   
398  $81\%$  and  $\text{O}_3/\text{UVC} = 93\%$ ; and (ii)  $L_{DC} = 19\%$ , and after  $\text{O}_3 = 83\%$  and  $\text{O}_3/\text{UVC} = 87\%$

399 (Table 1 and 2). Moreover, the COD values at the end of the Zahn-Wellens test were for  
400  $L_{NC}$  of 123 mg/L ( $O_3$ ) and 22 mg/L ( $O_3/UVC$ ), and for  $L_{DC}$  of 174 mg/L ( $O_3$ ) and 144  
401 mg/L ( $O_3/UVC$ ). In retrospect, except for  $O_3$  applied to  $L_{DC}$ , all treatment scenarios  
402 reached a final COD < 150 mg/L at the end of the biodegradability test.

### 403 3.2.2 Organic matter characterization

404 The coagulation stage reduced total fluorescence intensity by 61.5% and 60.3% for the  
405 nitrified and denitrified leachate, respectively (to discussion,  $L_{NC}$  will be assumed as  
406 resulting from the coagulation of  $L_N$ ). Concerning the fluorescence intensity by region  
407 (I to V), there was a decrease of 46%, 54%, 63%, 58% and 67%, from  $L_N$  to  $L_{NC}$ , and  
408 25%, 48%, 65%, 49% and 71%, from  $L_D$  to  $L_{DC}$  (Table SM-2). Despite the similarity in  
409 the fluorescence intensity decline (either total or by region, except for region I), as a  
410 result of the distinct biological and coagulation processes, different 3D-EEM patterns  
411 were obtained for the  $L_{NC}$  and  $L_{DC}$  effluents (Fig. 5 (b.1) and (b.2)). Aftab and Hur  
412 (2017) also reported higher removal of the humic-like (75%) versus the fulvic-like (~  
413 58%) acids for a coagulation treatment with an optimized dose of 300 mg  $Al^{3+}/L$ . The  
414 much lower removal of tyrosine-like protein (region I) was also verified in other studies  
415 (Aftab et al., 2020; Wassink et al., 2011), having been attributed to the smaller  
416 molecular size of these compounds.

### 417 **Insert Figure 5**

418 For  $L_{NC}$ , after  $O_3$  and  $O_3/UVC$  treatment, total fluorescence intensity was reduced by  
419 98.1% and 98.7%, respectively (Table SM-2). In both ozone-driven treatments, but  
420 more pronounced in  $O_3/UVC$ , the relative fluorescence intensity decreased (Fig. 5 (a.1))  
421 in the regions associated with fulvic- and humic-like matter (from 60.2% to 48.1% after  
422  $O_3$ , and to 38.9% after  $O_3/UVC$ ), and increased in the regions related to tyrosine-like  
423 protein and microbial by-products (from 14.3% to 35.5% after  $O_3$ , and to 44.6% after

424 O<sub>3</sub>/UVC). Those regions (I and IV) reflect bioavailable substrates (Henderson et al.,  
425 2009; Hudson et al., 2007), presenting a strong correlation with BOD<sub>5</sub> (Yang et al.,  
426 2013). These changes in DOM composition are in line with the results obtained in the  
427 Zahn-Wellens biodegradability tests. By applying coagulation/ozonation to treat  
428 membrane concentrates from landfill leachate, Chen et al. (2019) observed similar  
429 fluorescence trends and proposed that protein-like matter was produced as intermediates  
430 of the degradation of fulvic-like substances in the ozonation process, thereby producing  
431 an effluent suitable for a subsequent biological oxidation treatment stage.

432 In respect to L<sub>DC</sub>, total fluorescence intensity decreased 90.2% and 96.4% (Table SM-  
433 2), respectively, after O<sub>3</sub> and O<sub>3</sub>/UVC oxidation. Analysing the relative fluorescence  
434 (Fig. 5 (a.2)), the application of O<sub>3</sub> did not significantly change the proportions of  
435 fulvic- and humic-like acids (in fact, there was a slight increase from 53.5% to 56.7%).  
436 In turn, after O<sub>3</sub>/UVC treatment, not only the proportion of regions III and V in total  
437 fluorescence decreased, but also there was a significant increase (nearly two-fold) of the  
438 relative fluorescence intensity for tyrosine-like protein and microbial by-products.  
439 Again, these results are in good agreement with the observed by Chen et al. (2019) and  
440 with the biodegradability increase revealed by the Zahn-Wellens tests.

441 SEC-OCD chromatograms reveal once more the predominance of humic substances,  
442 with coagulation shifting humic peaks towards later retention time when compared to  
443 the respective bio-treated effluents (comparison between Fig. 3 (c) and Fig. 5 (c)). This  
444 indicates that coagulation not only reduced the organic carbon (peak area decrease of  
445 52.7% and 69.7%, respectively for L<sub>NC</sub> and L<sub>DC</sub>) but also removed the relatively high  
446 molecular weight humic matter, which is known to be carboxylic-rich. For L<sub>NC</sub> effluent,  
447 a peak area reduction of 87.2% and 91.1% were obtained for both ozone-driven  
448 treatments, with humic substances peak slightly changing to earlier retention time after

449 O<sub>3</sub>-only. In turn, for L<sub>DC</sub> treated by O<sub>3</sub>, not only the humic substances peak changed  
450 significantly to earlier retention time, suggesting higher molecular weight of the organic  
451 matter, but also the peak area only decreased 13.4%. The poor performance of O<sub>3</sub>  
452 treatment was partially overcome by its combination with UVC radiation, with a  
453 decrease of the humic substances peak area of 66.9% (but still 20% lower than those  
454 obtained for L<sub>NC</sub>).

### 455 *3.3. Treatment costs evaluation*

456 In view of simultaneous legal compliance for organic and nitrogen compounds, 6  
457 possible treatment train strategies were evaluated in terms of operating costs (Table 3):  
458 (1) bio-nitrification + O<sub>3</sub>/UVC + bio-denitrification; (2) bio-N removal + O<sub>3</sub>/UVC +  
459 bio-oxidation; (3,4) bio-nitrification + iron-acidic coagulation (no nitrites) + O<sub>3</sub> or  
460 O<sub>3</sub>/UVC + bio-denitrification; and (5,6) bio-N removal + aluminum-neutral coagulation  
461 + O<sub>3</sub> or O<sub>3</sub>/UVC + bio-oxidation).

### 462 **Insert Table 3**

463 Ozone-driven treatment stage is the most expensive among the tested unit treatment  
464 processes (Table 3). Coagulation substantially reduced the cost associated with the  
465 subsequent ozone process. Accordingly, the treatment trains without coagulation (no. 1  
466 and no. 2) were not economically viable. Regarding L<sub>N</sub>, if full nitrification (i.e., bio-  
467 oxidation of nitrite into nitrate) had occurred in the first biological reactor, it would be  
468 possible to expect a cost-reduction of ~40% for the O<sub>3</sub>-driven stage (to 20.5 €/m<sup>3</sup>, but  
469 still not economically viable). As coagulation promotes a significant reduction of the  
470 leachate organic load (~50%), the required ozone dose to achieve a biodegradable  
471 effluent, able to reach COD < 150 mg/L after a subsequent biological oxidation step, is  
472 significantly lower; thus operational costs are substantially reduced (treatment trains  
473 no.3 to 6). Considering the bio-coagulation pre-treatments, the strategies with L<sub>NC</sub>

474 presented the lowest operational costs (Table 3), particularly when the O<sub>3</sub>/UVC  
475 advanced oxidation process is applied (as it showed the highest pseudo-first-order rate  
476 constant for COD removal, Table 2). Nonetheless, knowing that only partial  
477 nitrification occurs in the 1<sup>st</sup> biological reactor and, from previous experiences using  
478 similar bio-nitrified leachate and coagulation conditions (Gomes et al., 2019a; Gomes et  
479 al., 2019b; Silva et al., 2017), at the end of the C/S stage, the leachate still presents part  
480 of the nitrogen content in the nitrite form (~ 70%). This is not desirable, as the presence  
481 of nitrites in downstream advanced stage oxidation will inevitably lead to an increase in  
482 costs (to 9.4 and 12.3 €/m<sup>3</sup>, for the O<sub>3</sub> process, considering leachate with 500 and 1000  
483 mg N/L of nitrite before coagulation). Therefore, the biological nitrogen removal should  
484 preferentially be performed at the first biological stage (strategies no. 5 and no. 6).  
485 Taking the reported costs (between 10 to 25 €/m<sup>3</sup> (Gomes, 2020)) of landfills in  
486 Portugal whose leachate treatment complies with the legal limits for direct discharge  
487 into the environment (by means of reverse osmosis process), treatment train strategy no.  
488 6 may be considered economically competitive. Furthermore, the problems arising from  
489 the concentrate that results from reverse osmosis are absent in ozone-based processes.

#### 490 **4. Conclusions**

491 The performance of O<sub>3</sub> and O<sub>3</sub>/UVC processes applied to urban mature leachate pre-  
492 treated with different technologies, the ability to comply with the legal limits for direct  
493 discharge into the environment, and the economic viability of different treatment trains,  
494 were evaluated in this work. In the absence of a coagulation stage, both bio-treated  
495 leachates (L<sub>N</sub> and L<sub>D</sub>) required an ozone dose ( $OD_T$ ) of 7.1 g O<sub>3</sub>/L combined with 59.5  
496 kJ/L of UVC radiation, to obtain a biodegradable effluent. However, it is not certain that  
497 after a subsequent biological treatment, the COD target value is attained, and the  
498 treatment cost exceeds 30 €/m<sup>3</sup>. Therefore, the inclusion of a chemical coagulation

499 process before the ozone-driven stage shows to be essential for the economic feasibility  
500 of a treatment train. The changes in DOM composition over the various treatment stages  
501 tested were consistent with the biodegradability presented by the respective effluents  
502 and showed that the degradation of fulvic- and humic-like matter by ozone-driven  
503 processes increases the content of the protein-like substance.

504 To deal with mature urban landfill leachate, among the various treatment train strategies  
505 proposed, the most suitable is as follows: (i) first biological stage for  
506 nitrification/denitrification (with the addition of an external carbon source), followed by  
507 (ii) coagulation, without pH adjustment and using 300 mg Al<sup>3+</sup>/L, and (iii) advanced  
508 oxidation by means of O<sub>3</sub>/UVC up to OD<sub>T</sub> of 2.1 g O<sub>3</sub>/L and 12.2 kJ<sub>UVC</sub>/L, prior to (iv)  
509 final biological oxidation. This sequence allows reaching a final effluent able to  
510 simultaneously comply with the legal discharge values for organic and nitrogen  
511 parameters, and for other parameters whose concentration increases with the addition of  
512 chemicals along the treatment train (such as sulfate ions and total iron). Furthermore,  
513 the operational cost for this treatment train is expected to be 8.9 €/m<sup>3</sup>, with O<sub>3</sub>/UVC  
514 process counting for 6.9 €/m<sup>3</sup>, which is reasonable considering the costs for current  
515 membrane technology (10 to 25 €/m<sup>3</sup>).

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699 **Legends**

700 **Figure 1** – Schematics of the experimental setup used for O<sub>3</sub> and O<sub>3</sub>/UVC treatment  
701 processes.

702 **Figure 2** – Evolution of (a) DOC removal, (b) COD removal, and (c) pH values as a  
703 function of treatment time for (.1) O<sub>3</sub> and (.2) O<sub>3</sub>/UVC treatments applied to the bio-  
704 treated leachates (■, ■■■) L<sub>N</sub> and (●, ■■■) L<sub>D</sub>, including (×) nitrite and (\*) nitrate  
705 concentrations, for L<sub>N</sub>.

706 **Figure 3** – (a) Relative fluorescence (■■■■■ - regions I to V), (b) 3D-EEM spectra,  
707 and (c) SEC-OCD chromatograms ((—) before and (—) after O<sub>3</sub>/UVC treatment) for  
708 the (.1) nitrified leachate – L<sub>N</sub> and (.2) nitrified-denitrified leachate – L<sub>D</sub>, and after (1'  
709 or .2') respective O<sub>3</sub>/UVC treatment.. (O<sub>3</sub>/UVC treatment conditions: V<sub>L</sub> = 1.5 L; Q<sub>O<sub>3</sub></sub> =  
710 0.1 L/min; [O<sub>3</sub>]<sub>inlet</sub> = 180 mg/L; ; t = 10h, Q<sub>UV</sub> = 2.48 J/s).

711 **Figure 4** – Evolution of (a) DOC removal, (b) COD removal, and (c) pH values as a  
712 function of treatment time for (.1) O<sub>3</sub> and (.2) O<sub>3</sub>/UVC treatments applied to the bio-  
713 coagulated leachates (■, ■■■) L<sub>N</sub> and (●, ■■■) L<sub>D</sub>.

714 **Figure 5** – (a) Relative fluorescence (■■■■■ - regions I to V), (b) 3D-EEM spectra,  
715 and (c) SEC-OCD chromatograms ((—) before and after (—) O<sub>3</sub> and (—) O<sub>3</sub>/UVC  
716 treatment) for the (.1) nitrified-coagulated leachate – L<sub>NC</sub> and (.2) nitrified-denitrified-  
717 coagulated leachate – L<sub>DC</sub>, and after respective (1' or 2') O<sub>3</sub> and (1'' or 2'') O<sub>3</sub>/UVC  
718 treatment. (coagulation conditions for L<sub>NC</sub>: [Fe<sup>3+</sup>] = 240 mg/L and pH ~4; coagulation  
719 conditions for L<sub>DC</sub>: [Al<sup>3+</sup>] = 300 mg/L and pH = 9.2; O<sub>3</sub> treatment conditions: V<sub>L</sub> = 1.5  
720 L; Q<sub>O<sub>3</sub></sub> = 0.1 L/min; [O<sub>3</sub>]<sub>inlet</sub> = 180 mg/L; t = 3h; O<sub>3</sub>/UVC treatment: same conditions as  
721 for O<sub>3</sub>, except for Q<sub>UV</sub> = 1.7 J/s).

722 **Table 1** – Physicochemical characterization of the landfill leachate after different pre-  
 723 treatment stages.

<b>Parameter</b>	<b>L<sub>N</sub></b>	<b>L<sub>D</sub></b>	<b>L<sub>NC</sub></b>	<b>L<sub>DC</sub></b>	<b>ELV<sup>a</sup></b>
<b>pH</b>	7.5	9.1	3.7	6.2	6.0 – 9.0
<b>DOC<sup>b</sup> (mg/L)</b>	956	1149	430	574	-
<b>DIC<sup>b</sup> (mg/L)</b>	89	464	7	143	-
<b>COD<sup>b</sup> (mg/L)</b>	3479	3577	1174	1635	150
<b>UV<sub>254</sub> (cm<sup>-1</sup>)</b>	27.1	24.9	6.8	4.1	-
<b>Color<sup>c</sup> (Pt-Co)</b>	320	280	49	26	Not visible
<b>NH<sub>4</sub><sup>+</sup> (mg N/L)</b>	< 0.1	< 0.1	< 0.1	< 0.1	8
<b>NO<sub>2</sub><sup>-</sup> (mg N/L)</b>	1034	< 0.1	< 0.1	< 0.1	-
<b>NO<sub>3</sub><sup>-</sup> (mg N/L)</b>	19	14	523	12	10
<b>Cl<sup>-</sup> (mg/L)</b>	2723	2480	2639	2630	-
<b>SO<sub>4</sub><sup>2-</sup> (mg/L)</b>	247	319	1721	1903	2000
<b>Al (mg/L)</b>	1.8	2.0	2.2	2.4	10
<b>Cr (mg/L)</b>	1.5	1.2	0.7	< 0.08	2.0
<b>Cu (mg/L)</b>	< 0.01	< 0.01	0.3	0.4	1.0
<b>Fe (mg/L)</b>	5.3	3.0	12.6	0.3	2.0
<b>Ni (mg/L)</b>	n.d.	n.d.	< 0.25	< 0.25	2.0
<b>Pb (mg/L)</b>	< 0.25	< 0.25	< 0.25	< 0.25	1.0
<b>Biodegradability (%)</b>	7	18	17	19	-

724 <sup>a</sup> Emission limit values (ELV) in Portugal for the discharge of wastewaters in receiving water bodies  
 725 (Decree-Law no 236/98).

726 <sup>b</sup> DOC – dissolved organic carbon; DIC – dissolved inorganic carbon; COD – chemical oxygen demand.

727 <sup>c</sup> Diluted 1:20.

728 **Table 2** – Main results and kinetic parameters, for DOC and COD degradation, of the O<sub>3</sub> and O<sub>3</sub>/UVC processes applied to bio-treated (L<sub>N</sub> and  
729 L<sub>D</sub>) and bio-coagulated (L<sub>NC</sub> and L<sub>DC</sub>) leachates.

Parameters	O <sub>3</sub>		O <sub>3</sub> /UVC		O <sub>3</sub>		O <sub>3</sub> /UVC		
	L <sub>N</sub>	L <sub>D</sub>	L <sub>N</sub>	L <sub>D</sub>	L <sub>NC</sub>	L <sub>DC</sub>	L <sub>NC</sub>	L <sub>DC</sub>	
<i>OD<sub>I</sub></i> <sup>a</sup> (g O <sub>3</sub> /L)	8.6	8.6	7.2	7.4	2.2	2.1	2.2	2.2	
<i>OD<sub>T</sub></i> <sup>a</sup> (g O <sub>3</sub> /L)	8.0	7.9	7.1	7.1	1.8	1.5	2.1	2.1	
DOC removal (%)	50.5	49.3	73.2	62.9	36.2	26.1	54.5	43.3	
COD removal (%)	64.8	68.7	79.4	82.9	53.1	36.4	69.9	60.0	
Color removal (%)	93.7	89.4	97.8	93.1	69.3	62.2	77.6	70.2	
Biodegradability (%)	51	69	73	80	81	83	93	87	
<b>DOC Degradation Kinetics</b>									
<i>k</i> <sup>b</sup> ×10 <sup>3</sup> (min <sup>-1</sup> )	2.0 ± 0.1	1.2 ± 0.1	4.7 ± 0.9	1.9 ± 0.2	2.5 ± 0.2	1.9 ± 0.2	4.2 ± 0.4	3.6 ± 0.4	
R <sup>2,c</sup>	0.997	0.995	0.990	0.994	0.996	0.996	0.991	0.992	
<i>t</i> <sup>d</sup> (min)	> 420	> 240	> 360	> 120	> 0	> 60	> 30	> 30	
<i>k</i> <sup>e</sup> ×10 <sup>-1</sup> (mg DOC/g OD <sub>T</sub> )	12.0 ± 0.9	8.5 ± 0.5	19 ± 1	10.4 ± 0.7	7.8 ± 0.8	8.4 ± 0.6	11.8 ± 0.8	12.7 ± 0.9	
R <sup>2,c</sup>	0.997	0.997	0.997	0.994	0.994	0.996	0.995	0.994	
<i>OD<sub>T</sub></i> <sup>d</sup> (g O <sub>3</sub> /L)	> 5.0	> 2.9	> 3.6	> 0	> 0	> 0	> 0.2	> 0.2	
<b>COD Degradation Kinetics</b>									
<i>k</i> <sup>b</sup> ×10 <sup>3</sup> (min <sup>-1</sup> )	1.7 ± 0.1	1.60 ± 0.08	2.7 ± 0.4	2.7 ± 0.2	4.2 ± 0.8	2.1 ± 0.4	6.5 ± 0.8	5.1 ± 0.5	
R <sup>2,c</sup>	0.991	0.994	0.994	0.995	0.990	0.991	0.993	0.996	
<i>t</i> <sup>d</sup> (min)	> 120	> 0	> 0	> 0	> 0	> 0	> 0	> 0	
<i>k</i> <sup>e</sup> ×10 <sup>-2</sup> (mg COD/g OD <sub>T</sub> )	3.0 ± 0.2	4.0 ± 0.3	2.2 ± 0.2	3.7 ± 0.2	2.7 ± 0.1	3.4 ± 0.6	3.3 ± 0.5	4.0 ± 0.6	4.6 ± 0.8
R <sup>2,c</sup>	0.994	0.997	0.994	0.998	0.998	0.996	0.993	0.995	0.991
<i>OD<sub>T</sub></i> <sup>d</sup> (g O <sub>3</sub> /L)	> 0	< 3.6	> 3.6	> 0	> 1.4	> 0	> 0	> 0	> 0

730 <sup>a</sup> Inlet (*OD<sub>I</sub>*) and transferred (*OD<sub>T</sub>*) ozone dose per liter of leachate.

731 <sup>b</sup> Pseudo-first-order rate constant for DOC and COD degradation in terms of reaction time.

732 <sup>c</sup> Coefficient of determination.

733 <sup>d</sup> Value or interval of time (*t*, min) or transferred ozone dose (*OD<sub>T</sub>*), from which the kinetic parameters were calculated.

734 <sup>e</sup> DOC and COD degradation reaction rates, expressed in terms of the transferred ozone dose (*OD<sub>T</sub>*), whose values correspond to the slopes of Fig. SM-1 (a.1) and (b.1), for  
735 L<sub>N</sub> and L<sub>D</sub>, and of Fig. SM-4 (a.1) and (b.1), for L<sub>NC</sub> and L<sub>DC</sub>.

736 **Table 3** – Operating costs estimation for the different treatment train strategies.

		Treatment train						
		Un	Bio <sub>NIT</sub> + O <sub>3</sub> /UVC + Bio <sub>DESN</sub>	Bio <sub>NIT</sub> /DESN + O <sub>3</sub> /UVC + Bio <sub>OXID</sub> .	Bio <sub>NIT</sub> + C/S + O <sub>3</sub> + Bio <sub>DESN</sub>	Bio <sub>NIT</sub> + C/S + O <sub>3</sub> /UVC + Bio <sub>DESN</sub>	Bio <sub>NIT</sub> /DESN + C/S + O <sub>3</sub> + Bio <sub>OXID</sub>	Bio <sub>NIT</sub> /DESN + C/S + O <sub>3</sub> /UVC + Bio <sub>OXID</sub>
1 <sup>st</sup> Bio. <sup>a</sup>	Aeration	€/m <sup>3</sup>	0.10	0.10	0.10	0.10	0.10	0.10
	Methanol	€/m <sup>3</sup>	-	0.57	-	-	0.57	0.57
	<b>(1) TOTAL</b>	<b>€/m<sup>3</sup></b>	<b>0.10</b>	<b>0.67</b>	<b>0.10</b>	<b>0.10</b>	<b>0.67</b>	<b>0.67</b>
Coagulation <sup>b</sup>	H <sub>2</sub> SO <sub>4</sub>	€/m <sup>3</sup>	-	-	0.08	0.08	-	-
	FeCl <sub>3</sub>	€/m <sup>3</sup>	-	-	0.42	0.42	-	-
	Al <sub>2</sub> (SO <sub>4</sub> ) <sub>3</sub>	€/m <sup>3</sup>	-	-	-	-	0.75	0.75
	Sludge	€/m <sup>3</sup>	-	-	0.48	0.48	0.48	0.48
	<b>(2) TOTAL</b>	<b>€/m<sup>3</sup></b>	<b>-</b>	<b>-</b>	<b>0.98</b>	<b>0.98</b>	<b>1.23</b>	<b>1.23</b>
O <sub>3</sub> -driven <sup>c</sup>	NaOH	€/m <sup>3</sup>	0.18	-	0.43	0.34	0.50	0.28
	O <sub>2</sub>	€/m <sup>3</sup>	4.07	3.75	1.16	0.78	1.95	0.96
	Energy	€/m <sup>3</sup>	29.4	27.1	5.01	4.58	8.42	5.65
	<b>(3) TOTAL</b>	<b>€/m<sup>3</sup></b>	<b>33.7</b>	<b>30.8</b>	<b>6.61</b>	<b>5.69</b>	<b>10.88</b>	<b>6.89</b>
2 <sup>nd</sup> Bio. <sup>d</sup>	Aeration	€/m <sup>3</sup>	0.03	0.06	0.03	0.03	0.06	0.06
	Methanol	€/m <sup>3</sup>	0.86	-	0.81	0.81	-	-
	<b>(4) TOTAL</b>	<b>€/m<sup>3</sup></b>	<b>0.89</b>	<b>0.06</b>	<b>0.84</b>	<b>0.84</b>	<b>0.06</b>	<b>0.06</b>
<b>(1) + (2) + (3) + (4)</b>			<b>34.6</b>	<b>31.6</b>	<b>8.5</b>	<b>7.6</b>	<b>12.8</b>	<b>8.6</b>

737 <sup>a</sup> The following theoretical values were assumed: 3.43 kg O<sub>2</sub> consumed per kg N-NO<sub>2</sub><sup>-</sup> formed (for nitrification) and 1.72 kg COD consumed per kg N-NO<sub>2</sub><sup>-</sup> reduced (for denitrification).

738 <sup>b</sup> Conditions: 240 mg Fe<sup>3+</sup>/L, pH ~ 4, requiring 0.42 L of H<sub>2</sub>SO<sub>4</sub> per m<sup>3</sup> of bio-nitrified leachate; and 300 mg Al<sup>3+</sup>/L without pH adjustment, for the nitrified/denitrified leachate.

739 <sup>c</sup> Conditions: initial pH adjusted to 9.0; Q<sub>g</sub> = 0.1 L/min; C<sub>O<sub>3</sub>,l</sub> = 180 mg O<sub>3</sub>/L.

740 <sup>d</sup> The following theoretical value was assumed: 2.89 kg COD per g N-NO<sub>3</sub><sup>-</sup> reduced (for denitrification).

**Figure 1**

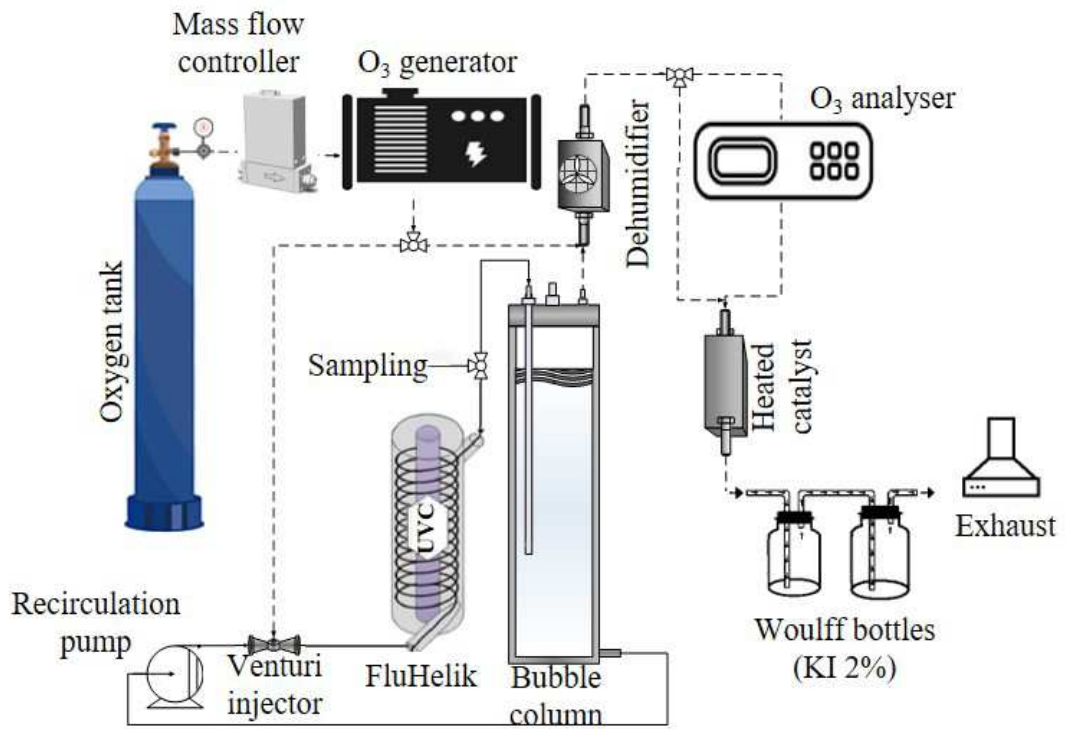
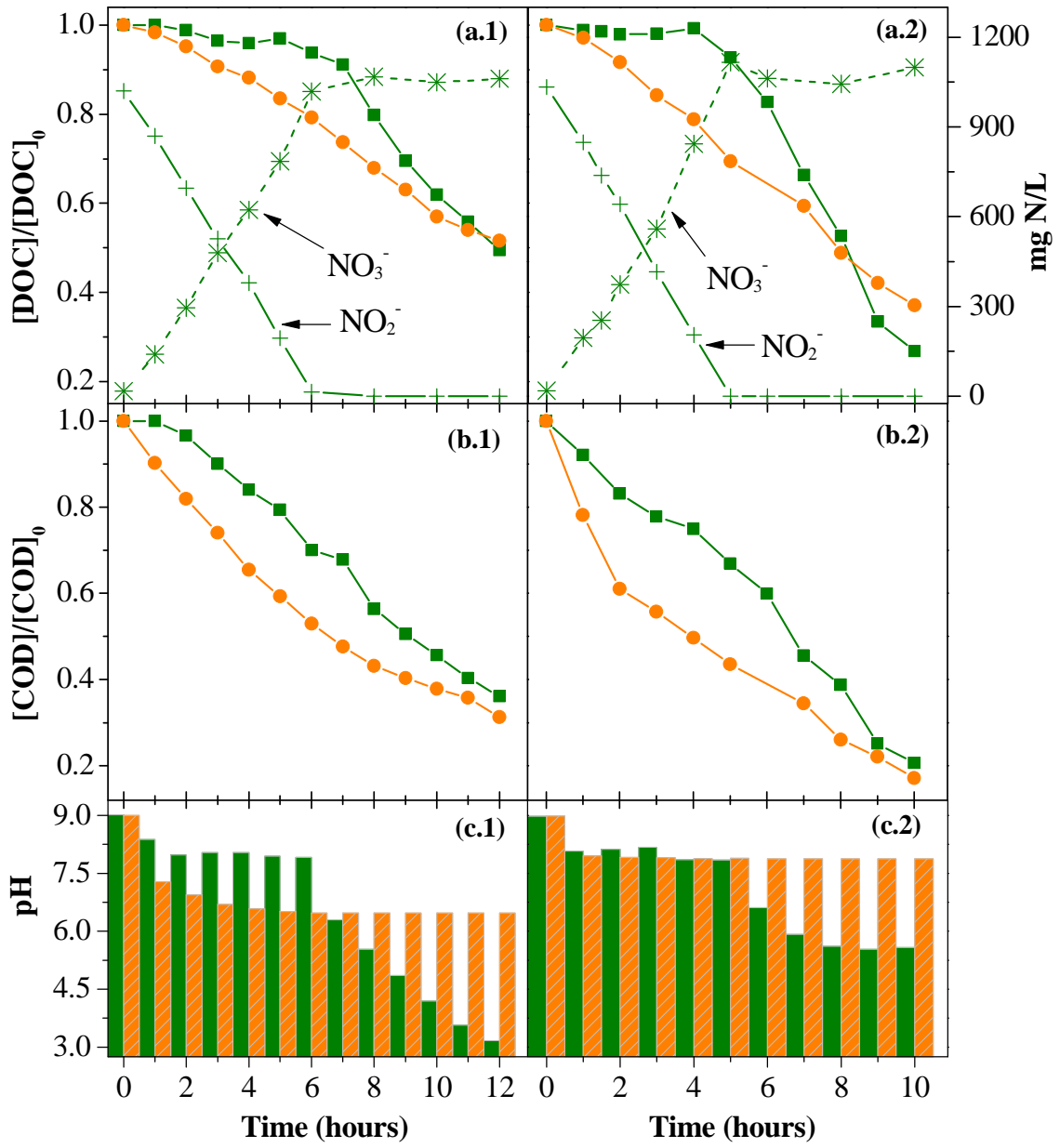
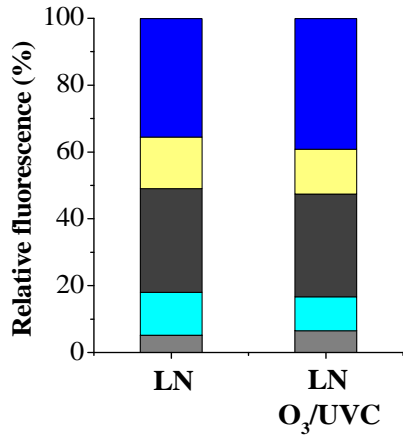


Figure 2

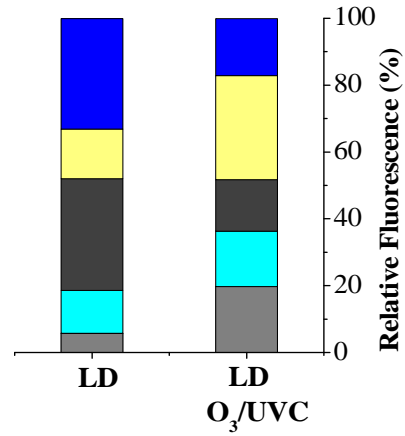


**Figure 3**

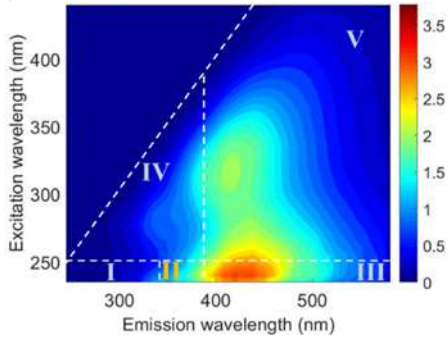
**(a.1)**



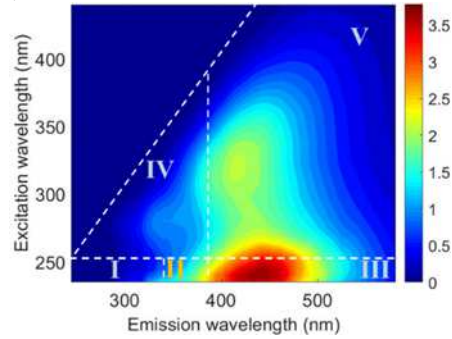
**(a.2)**



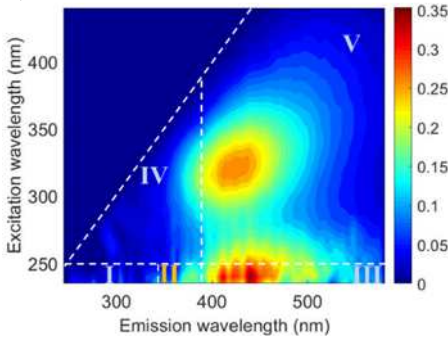
**(b.1)**



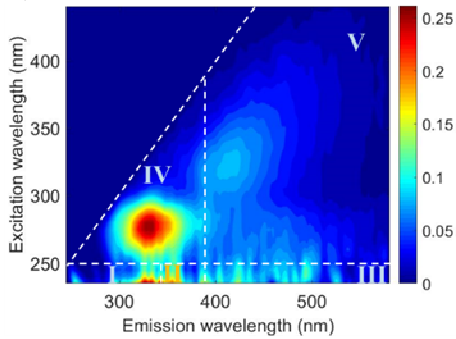
**(b.2)**



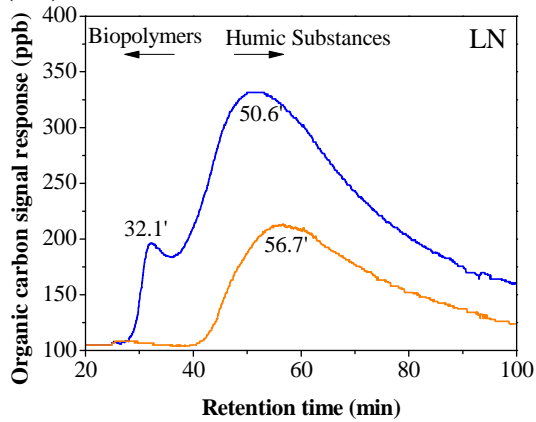
**(b.1')**



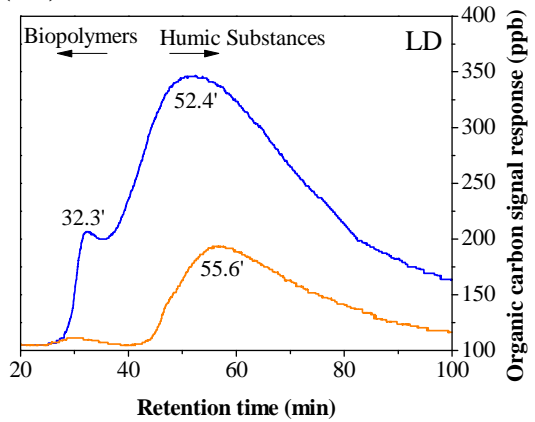
**(b.2')**



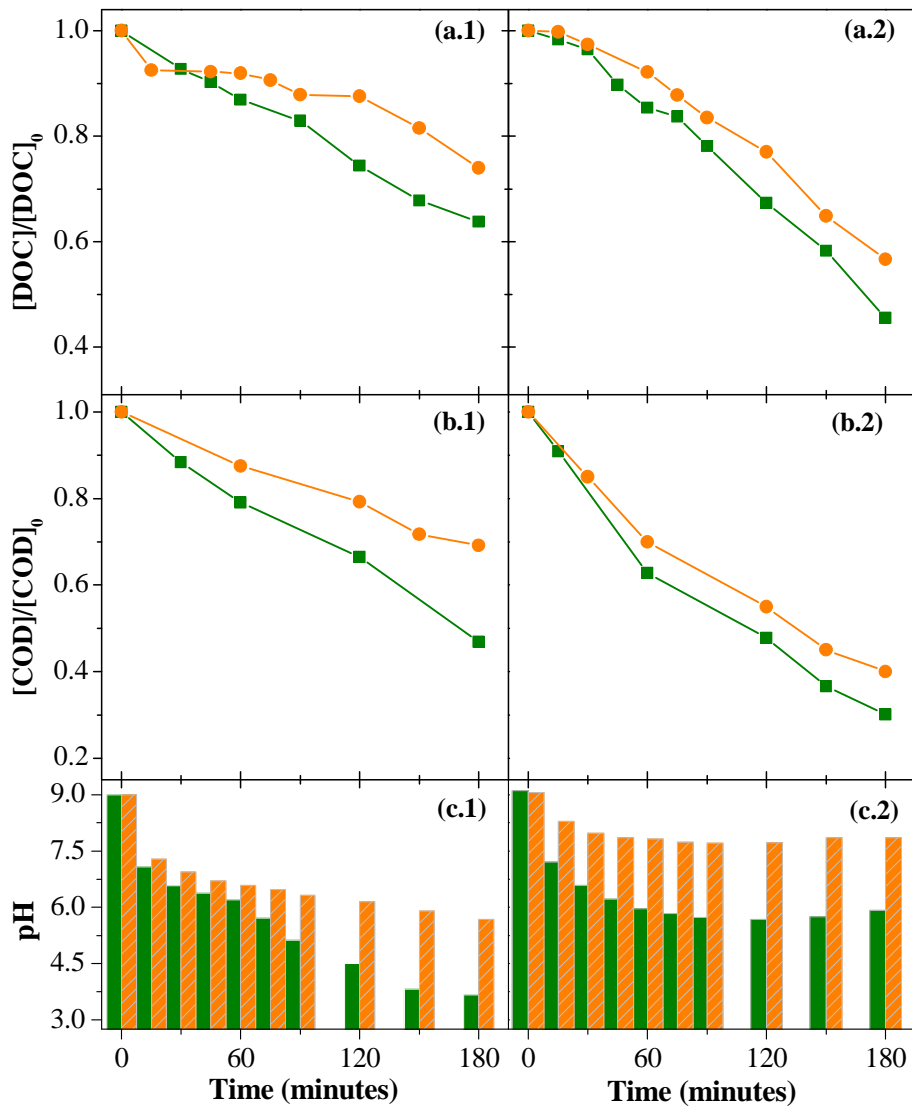
**(c.1)**



**(c.2)**



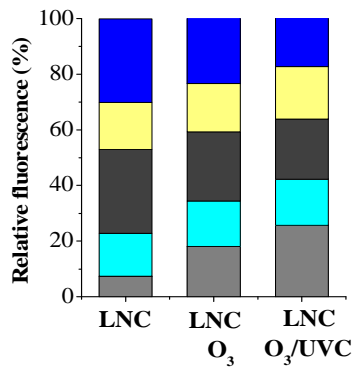
**Figure 4**



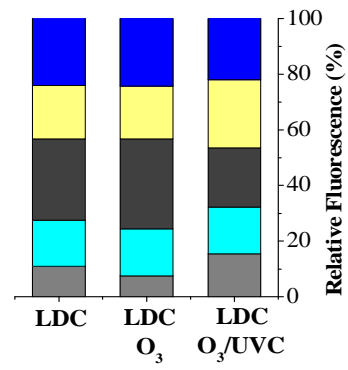


**Figure 5**

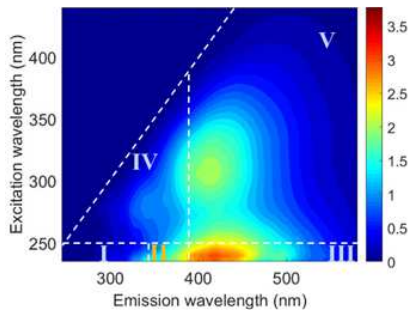
(a.1)



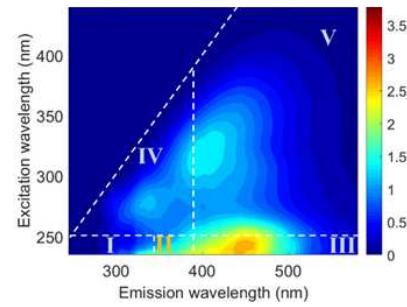
(a.2)



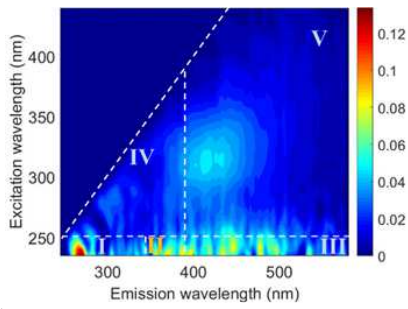
(b.1)



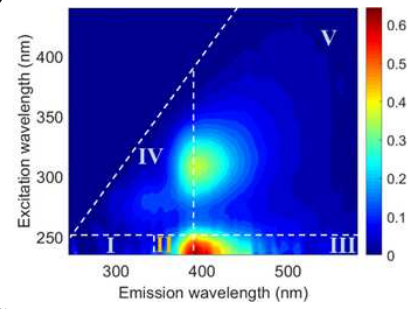
(b.2)



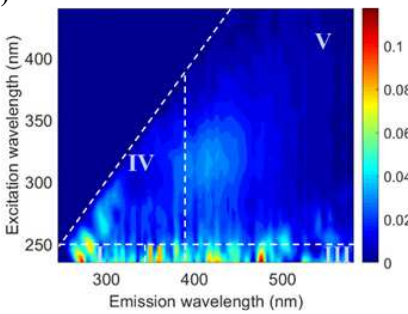
(b.1')



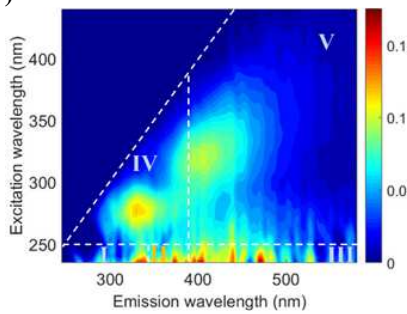
(b.2')



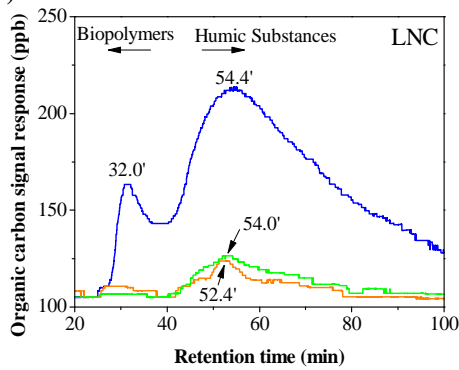
(b.1'')



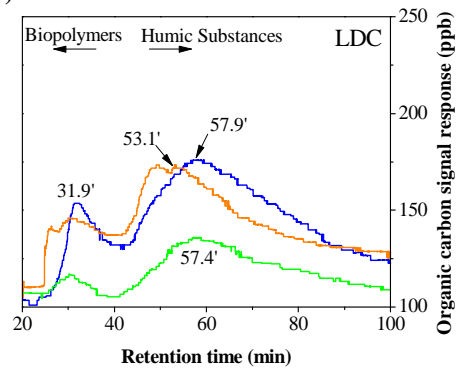
(b.2'')



(c.1)



(c.2)



## Highlights

- Fast  $O_3$  use to oxidize  $NO_2^-$  of the nitrified-leachate, thus hindering DOC decay.
- Coagulation selectively removed high molecular weight humic matter.
- Treatment train able to reach COD <150 mg/L, TN <15 mg/L and  $SO_4^{2-}$  <2000 mg/L.
- $O_3$ /UVC applied to  $L_{DC}$  represents 77.5% of the total operating cost (8.9 €/m<sup>3</sup>).

**Declaration of interests**

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

The authors declare the following financial interests/personal relationships which may be considered as potential competing interests: